**VOLUME 76** 

SEPARATE No. 14

# PROCEEDINGS

AMERICAN SOCIETY

OF

CIVIL ENGINEERS

**APRIL**, 1950



MATHEMATICAL ANALYSIS OF AN AERIAL SURVEY

By Lo-Ho

SURVEYING AND MAPPING DIVISION

Headquarters of the Society 33 W. 39th St. New York 18, N.Y.

PRICE \$0.50 PER COPY

The Society is not responsible for any statement made or opinion expressed in its publications

Published at Prince and Lemon Streets, Lancaster, Pa., by the American Society of Civil Engineers. Editorial and General Offices at 33 West Thirty-ninth Street, New York 18, N. Y. Reprints from this publication may be made on condition that the full title of paper, name of author, page reference, and date of publication by the Society are given.

Copyright 1950 by the American Society of Civil Engineers

Printed in the United States of America

## AMERICAN SOCIETY OF CIVIL ENGINEERS

Founded November 5, 1852

## PAPERS

## MATHEMATICAL ANALYSIS OF AN AERIAL SURVEY

By Lo-Ho1

## Synopsis

Aerial photographic methods are now widely used in topographic surveying, and in recent years efforts have been made to place them under mathematical control by analytical procedures of computation. In this field Earl Church,<sup>2</sup> Assoc. M. ASCE, and P. H. Underwood,<sup>3</sup> M. ASCE, are the leaders. The purpose of this paper is to present an analytical solution based on the direction-cosine principles of space geometry. The underlying mathematical principles are first developed and then applied to aerial survey problems. The methods of analysis and adjustment and the determination of the effect of the atmospheric refraction are new.

## DATA USED

The data immediately available in an aerial survey are the projections of the ground points on the negative plate, as transmitted through the objective lens; but the angles at the front nodal point, subtended by the separations of the objects at the instant of exposure, will be used in the following analysis. Furthermore, the positions of three or more ground points are also required, and these will be provided by the ground survey. The space angles at the front nodal point are assumed equal to their corresponding angles at the rear nodal point. The latter may be determined if the perpendicular distance from the rear nodal point to the plane of the negative plate, and the coordinates of the base of this perpendicular with respect to the fiducial axes on the negative plate, can be determined. An analysis of the geometrical relation between the rear nodal point and the fiducial axes is a part of this paper. The effect of atmospheric refraction is also analyzed; but the measurement of the coordinates of points in the negative by a comparator or other special instrument, and the

Note.—Written comments are invited for publication; the last discussion should be submitted by September 1, 1950.

<sup>&</sup>lt;sup>1</sup> Prof. of Surveying and Geodesy, Tangshan Eng. College, Tangshan, North China.

 <sup>&#</sup>x27;Manual of Photogrammetry,' Pitman Pub. Corp., New York, N. Y., and Chicago, Ill., 1944, p. 536.
 'Space Resection Problems in Photogrammetry,' by P. H. Underwood, Transactions, ASCE, Vol. 112, 1947, p. 921.

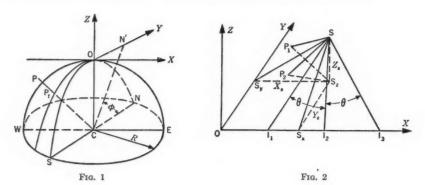
calibration of the camera to determine values of distortion, are not considered in the present paper.

## MATHEMATICAL PRINCIPLES

For convenience of reference, a summary of the mathematical principles directly involved in the following analysis will be presented first. Some well-known formulas are given without proof; but, for those not commonly known, the method of derivation will be outlined.

The method of adjustment described consists largely of the determination of corrections to be applied to computed or observed values. When the true values are connected by one or several conditional equations, the approximate relations between the corrections may be obtained by differentiation of such conditional equations, since the corrections to be applied are quite small in comparison with the values to be corrected. The resulting formulas will all be linear equations containing the corrections as unknowns and the differentials as exponents whose values may be found from the known values of the quantities. This general method will be used to derive many of the required equations.

Reference Axes.—The normal line through point O on the sphere OSWNE, Fig. 1, is adopted as the Z-axis; the north-south line through O, in the tangential plane of the sphere at point O, is the Y-axis; and the east-west line is the X-axis. The positive directions are all indicated by arrowheads as shown. The center of the sphere is designated point C, and its radius by R.



Normal Distance of a Point to the Sphere.—Let the coordinates of point P be  $X_p$ ,  $Y_p$ , and  $Z_p$ , and let a normal line from point P on the sphere be drawn to point  $P_f$  as shown in Fig. 1. Then

$$\overline{P_f P} = [X^2_p + Y^2_p + (Z_p + R)^2]^{\frac{1}{2}} - R$$

$$= Z_p + \frac{X^2_p + Y^2_p}{2(Z_p + R)} - \frac{(X^2_p + Y^2_p)^2}{8(Z_p + R)^3} + \cdots (1)$$

which may be used to determine the elevation of a point defined by coordinate distances.

Length of the Arc OPs.—The curved distance, OPs, on the sphere is given by

$$\overline{OP_f} = R \sin^{-1} \frac{(X^2_p + Y^2_p)^{\frac{1}{2}}}{R + \overline{P_f P}}.$$
 (2)

Eq. 2 may be used to determine the length of line OP at sea level.

Equation of a Normal Line.—The equation of the radial line PC is

$$\frac{X}{X_p} = \frac{Y}{Y_p} = \frac{Z+R}{Z_p+R}.$$
 (3)

which may be used to determine the equation of the plumb line from a point having known coordinates.

Coordinates of a Point from Its Elevation and Distance.—Let the azimuth of line OP, measured from the north point in clockwise direction be  $\alpha$ ; its curved distance OP<sub>f</sub> be s; and its elevation P<sub>f</sub>P be h. Then

$$X_p = (R+h)\sin\frac{s}{R}\sin\alpha....(4a)$$

$$Y_p = (R + h) \sin \frac{s}{R} \cos \alpha \dots (4b)$$

and

$$Z_p = (R+h)\cos\frac{s}{R} - R...$$
 (4c)

Eqs. 4 may be used to determine the coordinates of points located by a ground survey.

Azimuth Angle of a Line.—Let N' be a point on the Y-axis and let the vertical angle of the line CN' be  $\phi$  as shown in Fig. 1. Then the coordinates of N' are 0,  $R \cot \phi$ , and 0; and the equation of the plane containing points N', C, and P is

$$\begin{vmatrix} X & Y & Z & 1 \\ 0 & R \cot \phi & 0 & 1 \\ 0 & 0 & -R & 1 \\ X_p & Y_p & Z_p & 1 \end{vmatrix} = 0.$$
 (5)

Let  $P_1$  be another point with coordinates  $X_1$ ,  $Y_1$ , and  $Z_1$ . The equation of the plane containing points C, P, and  $P_1$  is found to be

$$\begin{vmatrix} X & Y & X & 1 \\ 0 & 0 & -R & 1 \\ X_p & Y_p & Z_p & 1 \\ X_1 & Y_1 & Z_1 & 1 \end{vmatrix} = 0....(6)$$

Eq. 5 can be used to determine the meridian plane through a given point; Eq. 6 gives the vertical plane through two given points; and the angle between the two planes is the azimuth of line  $PP_1$ , which may be determined from the equations of the two planes by the usual method.

Space Angles at a Point.—The coordinates  $OS_x$ ,  $OS_y$ , and  $S_sS$  of point S, Fig. 2, with respect to the rectangular axes are  $X_s$ ,  $Y_s$ , and  $Z_s$ , respectively. Similarly the coordinates of points  $P_1$  and  $P_2$  in the (XY)-plane are denoted by  $X_1$ ,  $Y_1$ ,  $X_2$ , and  $Y_2$ , respectively. Angle  $P_1SP_2$  may be determined from these coordinates thus:

$$\overline{S_z P_1} = \sqrt{(X_1 - X_s)^2 + (Y_1 - Y_s)^2}....(7a)$$

$$\overline{S_z P_2} = \sqrt{(X_2 - X_s)^2 + (Y_2 - Y_s)^2}.....(7b)$$

$$\overline{P_1P_2} = \sqrt{(X_2 - X_1)^2 + (Y_2 - Y_1)^2}.....(7c)$$

$$\overline{SP_1} = \sqrt{Z^2 + (\overline{S_2}P_1)^2} \dots (7d)$$

$$\overline{SP_2} = \sqrt{Z_s^2 + (\overline{S_z P_2})^2} \dots (7e)$$

and

$$\cos \angle \overline{P_1 S P_2} = \frac{(\overline{SP_1})^2 + (\overline{SP_2})^2 - (\overline{P_1 P_2})^2}{2 \overline{SP_1} \overline{SP_2}}....(7f)$$

Eqs. 7 can be used to determine the space angle at an exposure station. Lines  $SS_x$  and  $SS_y$  are perpendicular to axes OX and OY, respectively.

Coordinates in Terms of Axial Distances.—Let points  $I_1$ ,  $I_2$ , and  $I_3$ , Fig. 2, be points on the X-axis forming equal space angles  $I_1SI_2$  and  $I_2SI_3$  to be denoted by  $\theta$ ; and let the angle  $S_xSI_2$  be denoted by  $E_x$ . Then, from the relation,

$$\frac{\overline{SI_3}}{\overline{SI_1}} = \frac{\overline{I_2I_3}}{\overline{I_1I_2}} = \frac{\cos{(\theta - E_x)}}{\cos{(\theta + E_x)}}$$
, it may be found that

$$\tan E_x = \frac{\overline{I_2I_3} - \overline{I_1I_2}}{\overline{I_1I_3}} \cot \theta. \qquad (8)$$

Since  $\overline{S_xI_2} = \overline{SS_x} \tan E_x$  and  $\overline{S_xI_2} + \overline{I_2I_3} = \overline{SS_x} \tan (\theta + E_x)$ , it may be found that

$$\overline{SS_x} = \frac{\overline{I_2I_3}}{\tan (\theta + E_x) - \tan E_x}...(9)$$

and

$$\overline{S_x I_2} = \frac{\overline{I_2 I_3} \tan E_x}{\tan (\theta + E_x) - \tan E_x}....(10)$$

From Fig. 2, it is evident that

$$X_s = \overline{\mathrm{OI}_2} - \overline{\mathrm{S}_x \mathrm{I}_2}.....(11)$$

and

Formulas similar to Eqs. 8 to 11 may be derived, if three points on the Y-axis subtend equal space angles at point S. Eqs. 8 through 12 can be used to determine the perpendicular distance from the rear node of a lens to the plane of film emulsion, and the coordinates of the foot can be determined with respect to the fiducial axes.

Equation of Straight Line.—The coordinates of any point  $P_n$  will be denoted by  $X_n$ ,  $Y_n$ , and  $Z_n$  and their small corrections by  $x_n$ ,  $y_n$ , and  $z_n$ . Direction cosines of a line in the directions of X, Y, and Z will be denoted by L, M, and N and their small corrections by l, m, and n, respectively. The equation of line  $P_1P_2$  is

$$\frac{X - X_1}{L} = \frac{Y - Y_1}{M} = \frac{Z - Z_1}{N}.$$
 (13)

in which

$$L = \frac{X_2 - X_1}{r}....(14a)$$

$$M = \frac{Y_2 - Y_1}{r}....(14b)$$

and

$$N = \frac{Z_2 - Z_1}{r}....(14c)$$

In Eqs. 14,

$$r = \sqrt{(X_2 - X_1)^2 + (Y_2 - Y_1)^2 + (Z_2 - Z_1)^2}$$
....(15)

If small corrections  $x_1$ ,  $y_1$ , and  $z_1$  are added to the coordinates of point  $P_1$ , it can be found that

$$l = \frac{1}{r} \left[ - (1 - L^2) x_1 + L M y_1 + L N z_1 \right] \dots (16a)$$

$$m = \frac{1}{r} [L M x_1 - (1 - M^2) y_1 + M N z_1] \dots (16b)$$

and

$$n = \frac{1}{r} [L N x_1 + M N y_1 - (1 - N^2) z_1] \dots (16c)$$

Eq. 16c can be written

$$n = -\frac{L l + M m}{N}....(17)$$

Distance from a Point to a Line.—The perpendicular distance  $D_n$  from any point  $P_n$  to the line defined by Eq. 13 is given by

$$D^{2}_{n} = (X_{1} - X_{n})^{2} + (Y_{1} - Y_{n})^{2} + (Z_{1} - Z_{n})^{2} - [L(X_{1} - X_{n}) + M(Y_{1} - Y_{n}) + N(Z_{1} - Z_{n})]^{2}..(18)$$

Angle Between Two Intersecting Lines.—Let the direction cosines of two lines  $SP_1$  and  $SP_2$ , Fig. 3, be denoted by  $L_1$ ,  $M_1$ ,  $N_1$ ,  $L_2$ ,  $M_2$ , and  $N_2$ , respectively. The angle  $A_{12}$  included by the two lines ( $P_1S$  and  $SP_2$ , Fig. 3) is given by

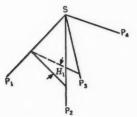


Fig. 3

$$\cos A_{12} = L_1 L_2 + M_1 M_2 + N_1 N_2 \dots (19a)$$

The correction  $a_{12}$  (in radians) of angle  $A_{12}$  due to small corrections of the direction cosines is given by

$$a_{12} = -\operatorname{cosec} A_{12} \left( L_2 l_1 + L_1 l_2 + M_2 m_1 + M_1 m_2 + N_2 n_1 + N_1 n_2 \right) \dots (19b)$$

If the corrections of the direction cosines are due to the corrections x, y, and z of the coordinates of point S alone, it may be found that

$$a_{12} = \frac{\operatorname{cosec} A_{12}}{r_1 r_2} \left\{ \left[ (r_1 L_1 + r_2 L_2) - (r_1 L_2 + r_2 L_1) \cos A_{12} \right] x + \left[ (r_1 M_1 + r_2 M_2) - (r_1 M_2 + r_2 M_1) \cos A_{12} \right] y + \left[ (r_1 N_1 + r_2 N_2) - (r_1 N_2 + r_2 N_1) \cos A_{12} \right] z \right\} \dots (20)$$

in which  $r_1$  and  $r_2$  are the original lengths of the lines  $SP_1$  and  $SP_2$ , respectively. Distance Between Two Lines.—The perpendicular distance between two

nonintersecting lines, 
$$\frac{X-X_1}{L_1}=\frac{Y-Y_1}{M_1}=\frac{Z-Z_1}{N_1}$$
 and  $\frac{X-X_2}{L_2}=\frac{Y-Y_2}{M_2}=\frac{Z-Z_2}{N_2}$ , is given by

$$d = \frac{\begin{vmatrix} X_1 - X_2 & Y_1 - Y_2 & Z_1 - Z_2 \\ L_1 & M_1 & N_1 \\ L_2 & M_2 & N_2 \end{vmatrix}}{\left[ (M_1 N_2 - M_2 N_1)^2 + (L_1 N_2 - L_2 N_1)^2 + (L_1 M_2 - L_2 M_1)^2 \right]^{\frac{1}{2}}}. (21a)$$

The condition of intersection of the two lines is controlled by

$$\begin{vmatrix} X_1 - X_2 & Y_1 - Y_2 & Z_1 - Z_2 \\ L_1 & M_1 & N_1 \\ L_2 & M_2 & N_2 \end{vmatrix} = 0 \dots (21b)$$

Small corrections  $x_1$ ,  $x_2$ ,  $y_1$ ,  $y_2$ ,  $z_1$ ,  $z_2$ ,  $l_1$ ,  $l_2$ ,  $m_1$ ,  $m_2$ ,  $n_1$ , and  $n_2$  to be added to the coordinates and the direction cosines of two nearly intersecting lines to make them intersect are connected by

$$\begin{vmatrix} X_{1} - X_{2} & Y_{1} - Y_{2} & Z_{1} - Z_{2} \\ L_{1} & M_{1} & N_{1} \\ L_{2} & M_{2} & N_{2} \end{vmatrix} + \begin{vmatrix} X_{1} - X_{2} & Y_{1} - Y_{2} & z_{1} - z_{2} \\ L_{1} & M_{1} & n_{1} \\ L_{2} & M_{2} & n_{2} \end{vmatrix} + \begin{vmatrix} X_{1} - X_{2} & Y_{1} - Y_{2} & Z_{1} - Z_{2} \\ L_{1} & M_{1} & N_{1} \\ L_{2} & m_{2} & N_{2} \end{vmatrix} + \begin{vmatrix} x_{1} - x_{2} & Y_{1} - Y_{2} & Z_{1} - Z_{2} \\ l_{1} & M_{1} & N_{1} \\ l_{2} & M_{2} & N_{2} \end{vmatrix} = 0..(22)$$

Eq. 22 was obtained by neglecting small terms involving powers of the corrections, which powers are higher than the first.

The coordinates  $X_m$ ,  $Y_m$ , and  $Z_m$  of the middle point  $P_m$  of the line perpendicular to the foregoing two lines may be found to be

$$X_{m} = \frac{1}{2} (X_{1} + X_{2})$$

$$= \frac{\begin{vmatrix} X_{1} - X_{2} & Y_{1} - Y_{2} & Z_{1} - Z_{2} \\ 2 L_{1} L_{2} & L_{1} M_{2} + L_{2} M_{1} & L_{1} N_{2} + L_{2} N_{1} \\ M_{1} N_{2} - M_{2} N_{1} & L_{2} N_{1} - L_{1} N_{2} & L_{1} M_{2} - L_{2} M_{1} \end{vmatrix}}{2 \left[ (M_{1} N_{2} - M_{2} N_{1})^{2} + (L_{2} N_{1} - L_{1} N_{2})^{2} + (L_{1} M_{2} - L_{2} M_{1})^{2} \right]} ..(23a)$$

$$Y_{m} = \frac{1}{2} (Y_{1} + Y_{2})$$

$$= \frac{\begin{vmatrix} X_{1} - X_{2} & Y_{1} - Y_{2} & Z_{1} - Z_{2} \\ L_{1} M_{2} + L_{2} M_{1} & 2 M_{1} M_{2} & M_{1} N_{2} + M_{2} N_{1} \\ M_{1} N_{2} - M_{2} N_{1} & L_{2} N_{1} - L_{1} N_{2} & L_{1} M_{2} - L_{2} M_{1} \end{vmatrix}}{2 \left[ (M_{1} N_{2} - M_{2} N_{1})^{2} + (L_{2} N_{1} - L_{1} N_{2})^{2} + (L_{1} M_{2} - L_{2} M_{1})^{2} \right]} . . (23b)$$

and

$$Z_{m} = \frac{1}{2} (Z_{1} + Z_{2})$$

$$= \frac{\begin{vmatrix} X_{1} - X_{2} & Y_{1} - Y_{2} & Z_{1} - Z_{2} \\ L_{1} N_{2} + L_{2} N_{1} & M_{1} N_{2} + M_{2} N_{1} & 2 N_{1} N_{2} \\ M_{1} N_{2} - M_{2} N_{1} & L_{2} N_{1} - L_{1} N_{2} & L_{1} M_{2} - L_{2} M_{1} \end{vmatrix}}{2 \left[ (M_{1} N_{2} - M_{2} N_{1})^{2} + (L_{2} N_{1} - L_{1} N_{2})^{2} + (L_{1} M_{2} - L_{2} M_{1})^{2} \right]} . (23c)$$

which may be used to determine the most probable point of intersection of two observed lines that should meet at a point if there were no errors of observation.

Three Concurrent Lines.—Let the space angles between the three concurrent lines  $SP_1$ ,  $SP_2$ , and  $SP_3$ , as shown in Fig. 3, be denoted by  $A_{12}$ ,  $A_{13}$ , and  $A_{23}$ , respectively; and let  $H_1$  denote the dihedral angle between the two planes  $SP_1P_2$  and  $SP_1P_3$ . By the law of cosines in spherical trigonometry,

$$\cos H_1 = \frac{\cos A_{23} - \cos A_{12} \cos A_{13}}{\sin A_{12} \sin A_{13}}....(24)$$

If  $H_1$  remains unchanged, but  $A_{12}$  and  $A_{13}$  are corrected by  $a_{12} = -\rho \tan A_{12}$  and  $a_{13} = -\rho \tan A_{13}$ , respectively, the correction to be applied to  $A_{23}$  is given by

$$a_{23} = \rho \operatorname{cosec} A_{23} \left( 2 \operatorname{cos} A_{23} - \frac{\operatorname{cos} A_{13}}{\operatorname{cos} A_{12}} - \frac{\operatorname{cos} A_{12}}{\operatorname{cos} A_{13}} \right) \dots (25)$$

When the direction cosines  $L_1$ ,  $M_1$ ,  $N_1$ ,  $L_2$ ,  $M_2$ , and  $N_2$  and the space angles  $A_{13}$  and  $A_{23}$  are known, the direction cosines of line SP<sub>3</sub> may be found by solving the equations:

$$\cos A_{13} = L_1 L_3 + M_1 M_3 + N_1 N_3 \dots (26a)$$

$$\cos A_{23} = L_2 L_3 + M_2 M_3 + N_2 N_3 \dots (26b)$$

and

$$L^{2}_{3} + M^{2}_{3} + N^{2}_{3} = 1 \dots (26c)$$

From Eqs. 26a and 26b, the magnitudes  $L_3$  and  $M_3$  may be found in terms of  $N_3$ ; and, by substitution in Eq. 26c, a quadratic is obtained which may be solved for the root  $N_3$ .

If points  $P_1$  and  $P_2$  are fixed in position, if point S is moved a little to a new position having (X + x), (Y + y), and (Z + z) as its new coordinates, and if the space angles  $A_{13}$  and  $A_{23}$  are given small corrections  $a_{13}$  and  $a_{23}$ , respectively, the corrections  $l_1$ ,  $m_1$ ,  $n_1$ ,  $l_2$ ,  $m_2$ , and  $n_2$  may be found in terms of x, y, and z by Eqs. 16 and  $l_3$ ,  $m_3$ , and  $n_3$  may be found in terms of x, y, and z through

the solution of the equations:

$$L_1 l_3 + M_1 m_3 + N_1 n_3 = -\sin A_{13} a_{13} - L_3 l_1 - M_3 m_1 - N_3 n_1 \dots (27a)$$

$$L_2 l_3 + M_2 m_3 + N_2 n_3 = -\sin A_{23} a_{23} - L_3 l_2 - M_3 m_2 - N_3 n_2$$
. (27b)

and

which are obtained by differentiating Eqs. 26.

Four Concurrent Lines.—Let the space angles between the four lines  $SP_1$ ,  $SP_2$ ,  $SP_3$ , and  $SP_4$ , in Fig. 3, be denoted by  $A_{12}$ ,  $A_{13}$ ,  $A_{14}$ ,  $A_{23}$ ,  $A_{24}$ , and  $A_{34}$ . Since the dihedral angles between the planes  $SP_1P_2$ ,  $SP_1P_3$ , and  $SP_1P_4$  can be expressed in terms of the space angles by Eq. 24 and the sum or difference of two is equal to the third, by equating the cosine of the one to the cosine of the sum of the other two, it is found that

$$\cos A_{34} = \cos A_{13} \cos A_{14} + \frac{K_{123} K_{124}}{\sin^2 A_{12}} + \frac{(\cos A_{23} - \cos A_{12} \cos A_{13}) (\cos A_{24} - \cos A_{12} \cos A_{14})}{\sin^2 A_{12}}..(28)$$

in which

$$K_{123} = (1 + 2\cos A_{12}\cos A_{13}\cos A_{23} - \cos^2 A_{12} - \cos^2 A_{13} - \cos^2 A_{23})^{\frac{1}{2}}..(29a)$$

and

$$K_{124} = (1 + 2\cos A_{12}\cos A_{14}\cos A_{24} - \cos A_{12} - \cos^2 A_{14} - \cos^2 A_{24})^{\frac{1}{2}}..(29b)$$

The second term on the right side of Eq. 28 will have a negative sign when points P<sub>3</sub> and P<sub>4</sub> are on the opposite sides of plane SP<sub>1</sub>P<sub>2</sub>. The small corrections of the six space angles are connected by

$$a_{34} = c_{12} a_{12} + c_{13} a_{13} + c_{14} a_{14} + c_{23} a_{23} + c_{24} a_{24} \dots (30)$$

in which the differential coefficients are

$$c_{12} = 2 \cot A_{12} \cot A_{34} - \frac{K_{124} (\cos A_{12} - \cos A_{13} \cos A_{23})}{K_{123} \sin A_{12} \sin A_{24}} - \frac{K_{123} (\cos A_{12} - \cos A_{14} \cos A_{24})}{K_{124} \sin A_{12} \sin A_{34}} - \frac{\cos A_{14} \cos A_{23} + \cos A_{13} \cos A_{24}}{\sin A_{12} \sin A_{34}}..(31a)$$

$$c_{13} = \frac{\sin A_{13} (\cos A_{14} - \cos A_{12} \cos A_{24})}{\sin^2 A_{12} \sin A_{34}} - \frac{K_{124} \sin A_{13} (\cos A_{13} - \cos A_{12} \cos A_{23})}{K_{123} \sin^2 A_{12} \sin A_{34}}..(31b)$$

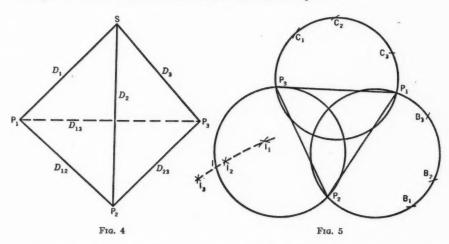
$$c_{14} = \frac{\sin A_{14} (\cos A_{18} - \cos A_{12} \cos A_{23})}{\sin^2 A_{12} \sin A_{34}} - \frac{K_{123} \sin A_{14} (\cos A_{14} - \cos A_{12} \cos A_{24})}{K_{124} \sin^2 A_{12} \sin A_{34}} ...(31c)$$

$$c_{23} = \frac{\sin A_{23} (\cos A_{24} - \cos A_{12} \cos A_{14})}{\sin^2 A_{12} \sin A_{34}} - \frac{K_{124} \sin A_{23} (\cos A_{23} - \cos A_{12} \cos A_{13})}{K_{123} \sin^2 A_{12} \sin A_{34}} ...(31d)$$

and

$$c_{24} = \frac{\sin A_{24} (\cos A_{23} - \cos A_{12} \cos A_{13})}{\sin^2 A_{12} \sin A_{34}} - \frac{K_{123} \sin A_{24} (\cos A_{24} - \cos A_{12} \cos A_{14})}{K_{124} \sin^2 A_{12} \sin A_{34}}..(31e)$$

If the angles  $A_{12}$ ,  $A_{13}$ ,  $A_{14}$ ,  $A_{23}$ ,  $A_{24}$ , and  $A_{34}$  are determined by the coordinates of points S,  $P_1$ ,  $P_2$ ,  $P_3$ , and  $P_4$ , the value of  $A_{34}$ , obtained from the triangle  $SP_3P_4$ , will be identical with that determined from Eq. 28.



If the conditional formula (Eq. 28) is satisfied, the direction cosines  $L_4$ ,  $M_4$ , and  $N_4$  of line  $SP_4$  are determined from the values of the direction cosines in the other three lines by the simultaneous linear equations:

$$\cos A_{14} = L_1 L_4 + M_1 M_4 + N_1 N_4 \dots (32a)$$

$$\cos A_{24} = L_2 L_4 + M_2 M_4 + N_2 N_4 \dots (32b)$$

and

$$\cos A_{34} = L_3 L_4 + M_3 M_4 + N_3 N_4 \dots (32c)$$

Angles and Sides of a Tetrahedron.—The space angles of the tetrahedron at the vertex S, Fig. 4, are denoted by  $A_{12}$ ,  $A_{13}$ , and  $A_{23}$ ; and the lengths of the sides, SP<sub>1</sub>, SP<sub>2</sub>, P<sub>1</sub>P<sub>2</sub>, P<sub>1</sub>P<sub>3</sub>, and P<sub>2</sub>P<sub>3</sub>, are denoted by  $D_1$ ,  $D_2$ ,  $D_3$ ,  $D_{12}$ ,  $D_{13}$ ,

and  $D_{23}$ , respectively. The relations between these angles and sides are

$$D^{2}_{12} = D^{2}_{1} + D^{2}_{2} - 2 D_{1} D_{2} \cos A_{12} \dots (33a)$$

$$D^{2}_{13} = D^{2}_{1} + D^{2}_{3} - 2 D_{1} D_{3} \cos A_{13} \dots (33b)$$

and

$$D^{2}_{23} = D^{2}_{2} + D^{2}_{3} - 2 D_{2} D_{3} \cos A_{23} \dots (33c)$$

which are usually used to determine  $D_1$ ,  $D_2$ , and  $D_3$  from the known values of  $A_{12}$ ,  $A_{13}$ ,  $A_{23}$ ,  $D_{12}$ ,  $D_{13}$ , and  $D_{23}$ . The small corrections  $d_1$ ,  $d_2$ , and  $d_3$  of  $D_1$ ,  $D_2$ , and  $D_3$ , respectively, are connected by

$$d_2 = \frac{D_1 - D_2 \cos A_{12}}{D_1 \cos A_{12} - D_2} d_1.$$
 (34a)

$$d_3 = \frac{D_1 - D_3 \cos A_{13}}{D_1 \cos A_{13} - D_3} d_1.$$
 (34b)

and

$$d_2 = \frac{D_3 - D_2 \cos A_{23}}{D_3 \cos A_{23} - D_2} d_3.$$
 (34c)

Combining Eqs. 34b and 34c,

$$d_2 = \frac{D_1 - D_3 \cos A_{13}}{D_1 \cos A_{13} - D_3} \times \frac{D_3 - D_2 \cos A_{23}}{D_3 \cos A_{23} - D_2} d_1.$$
 (35)

Geometrical Solution of a Tetrahedron.—To determine  $D_1$ ,  $D_2$ , and  $D_3$  geometrically, first construct a plane triangle  $P_1P_2P_3$  to scale and draw circular arcs on its three sides to contain the vertex angles  $A_{12}$ ,  $A_{13}$ , and  $A_{23}$ , respectively, as shown in Fig. 5. Then with an estimated value of  $D_1$  as radius and point  $P_1$  as a center draw a circular arc to intersect the circles on either side of point  $P_1$  at points  $P_1$  and  $P_2$  and  $P_3$  are radius, draw the intersecting arcs at  $P_1$ , Fig. 5, using points  $P_2$  and  $P_3$ , respectively, as centers. If point  $P_3$  falls within the third circle as shown, the assumed value of  $P_3$  is probably too long.

Use a revised value of  $D_1$  to repeat the same operation, thus determining another point  $I_2$ , and, if necessary, a third point  $I_3$ . Let the smooth curve through points  $I_1$ ,  $I_2$ , and  $I_3$  intersect the outer arc of the third circle at point I, Fig. 5. The distances  $P_2I$  and  $P_3I$  are the required values of  $D_2$  and  $D_3$  respectively. With line  $P_2I$  representing the value of  $D_2$ , it is an easy matter to determine  $D_1$ . The foregoing method can be used to determine  $D_1$ ,  $D_2$ , and  $D_3$  to three decimal places.

Analytical Solution of a Tetrahedron.—The approximate value  $D'_1$  determined by the geometrical method is used for  $D_1$  in determining the value  $D'_2$  of  $D_2$  by Eq. 33a and the value  $D'_3$  of  $D_3$  by Eq. 33b. Then  $D'_3$  is introduced as the value of  $D_3$  to find the value  $D''_2$  of  $D_2$  by Eq. 33c, and finally the correction  $d_1$  is found by

$$D'_{2} + \frac{D'_{1} - D'_{2} \cos A_{12}}{D'_{1} \cos A_{12} - D'_{2}} d_{1} = D''_{2} + \frac{D'_{1} - D'_{3} \cos A_{13}}{D'_{1} \cos A_{13} - D'_{3}} \times \frac{D'_{3} - D''_{2} \cos A_{23}}{D'_{3} \cos A_{23} - D''_{2}} d_{1}...(36)$$

Eq. 36, solved for  $d_1$ , becomes

$$d_{1} = \frac{D'_{2} - D''_{2}}{\frac{D'_{1} - D'_{3} \cos A_{13}}{D'_{1} \cos A_{13} - D'_{3}} \times \frac{D'_{3} - D''_{2} \cos A_{23}}{\frac{D'_{3} \cos A_{23} - D''_{2}}{D'_{1} \cos A_{12} - D'_{2}}} - \frac{D'_{1} - D'_{2} \cos A_{12}}{\frac{D'_{1} - D'_{2} \cos A_{12}}{D'_{1} \cos A_{12} - D'_{2}}}.$$
 (37)

The values obtainable from the foregoing process of calculation are

$$D_1 = D'_1 + d_1 \dots (38a)$$

$$D_2 = D'_2 + \frac{D'_1 - D'_2 \cos A_{12}}{D'_1 \cos A_{12} - D'_2} d_1.$$
 (38b)

and

$$D_3 = D'_3 + \frac{D'_1 - D'_3 \cos A_{13}}{D'_1 \cos A_{13} - D'_3} d_1....(38c)$$

More accurate values can be found by using the improved value of  $D_1$  to repeat the process.

If the coordinates  $X_1$ ,  $Y_1$ ,  $Z_1$ ,  $X_2$ ,  $Y_2$ ,  $Z_2$ ,  $X_3$ ,  $Y_3$ , and  $Z_3$  of points  $P_1$ ,  $P_2$ , and  $P_3$ , and the lengths of the sides  $D_1$ ,  $D_2$ , and  $D_2$  are known, coordinates X, Y, and Z of point S are found by solving the equations:

$$D^{2}_{1} = (X - X_{1})^{2} + (Y - Y_{1})^{2} + (Z - Z_{1})^{2} \dots (39a)$$

$$D^{2}_{2} = (X - X_{2})^{2} + (Y - Y_{2})^{2} + (Z - Z_{2})^{2} \dots (39b)$$

and

$$D^{2}_{3} = (X - X_{3})^{2} + (Y - Y_{3})^{2} + (Z - Z_{3})^{2} \dots (39c)$$

from which it follows that

$$2(X_1 - X_2)X + 2(Y_1 - Y_2)Y + 2(Z_1 - Z_2)Z$$
  
=  $X^{2_1} + Y^{2_1} + Z^{2_1} - X^{2_2} - Y^{2_2} - Z^{2_2} - D^{2_1} + D^{2_2}$ . (40a)

and

$$2(X_1 - X_3) X + 2(Y_1 - Y_3) Y + 2(Z_1 - Z_3) Z$$
  
=  $X^{2_1} + Y^{2_1} + Z^{2_1} - X^{2_3} - Y^{2_3} - Z^{2_3} - D^{2_1} + D^{2_3}$ . (40b)

Thus, X and Y are found in terms of Z in the following form:

$$X = c_1 Z + k_1 \dots (41a)$$

and

in which  $c_1$ ,  $c_2$ ,  $k_1$ , and  $k_2$  are numerical constants. Substituting the equivalents of X and Y into one of the original equations and solving it, two values of Z are obtained which correspond to two positions of point S, one above and one below the base  $P_1P_2P_3$ .

Refraction of Air.—According to the theory of atmospheric refraction, the total change of direction of the light ray from its position at infinity to point  $S_1$  in the atmosphere, as shown in Fig. 6, is given approximately by

$$C_1 = (g_1 - 1) \tan \zeta_1 \dots (42a)$$

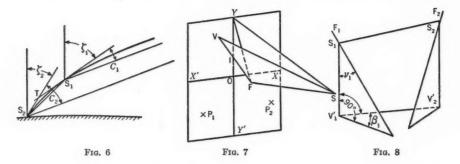
in which  $C_1$  is the angle between the tangent line of the path of light at point  $S_1$  and its original direction;  $\zeta_1$  is the angle between the tangent and the plumb line; and  $g_1$  is the refractive index of air at point  $S_1$ . Similarly, at another point  $S_2$  on the same path of light,

$$C_2 = (g_2 - 1) \tan \zeta_2 \dots (42b)$$

The change of direction of the curve from points S1 to S2 is equal to

$$C_{12} = (g_2 - 1) \tan \zeta_2 - (g_1 - 1) \tan \zeta_1 \dots (42c)$$

If the path of the light ray is very flat (as is actually the case),  $\zeta_1$  and  $\zeta_2$  may be



considered as equal, and, therefore,

$$C_{12} = (g_2 - g_1) \tan \zeta_1 \dots (42d)$$

In that case it may be reasonably assumed that the angle between the chord  $S_1S_2$  and the tangent at point  $S_1$  is proportional to  $C_{12}$ . Consequently,

in which  $\rho$  is an unknown constant whose value mainly depends on the height of point  $S_1$  above point  $S_2$ . The negative sign is used to account for the fact that the zenith distance of the direction of the chord  $S_2S_1$  is smaller than  $\zeta_1$ , the direction of the tangent  $TS_1$ . Since angle  $TS_1S_2$  is always less than the astronomical refraction in numerical value, the value of  $\rho$  in minutes of arc is less than 1, if the zenith distance does not exceed 75°.

#### PROBLEMS IN AERIAL SURVEY

Camera Constants.—The relation between the rear nodal point and the fiducial axes on the negative plate is essentially as shown in Fig. 2. If the images  $I_1$ ,  $I_2$ , and  $I_3$  of three objects which are at equal angles from each other appear on both fiducial axes, the actual angles  $\theta$  at the exposure station between the real objects are measured with a theodolite, the lengths  $OI_1$ ,  $OI_2$ , and  $OI_3$  along the fiducial axes on the negative plate can be determined accurately (corrected for distortion if necessary), and the three coordinates of the rear nodal point may be calculated by Eqs. 8 through 12. On account of the errors

of measurement and distortion, the values determined from different sets of data may be slightly different.

When the coordinates of the rear nodal point are exactly known, the space angle at the node subtended by any two image points on the negative plate can be calculated if the coordinate distances of the points can be measured and corrected for distortion.

Position of Exposure Station.—When the coordinate system is so assumed that its (XY)-plane is tangent to the spheroid at a point of given latitude and longitude and the point of tangency is used as origin, the coordinates of a point defined by elevation, direction, and distance from the origin may be calculated by Eqs. 4. The straight distance between two points of known coordinates can be computed easily. If the space angles subtended at the exposure station by three points of known positions can be determined, the position of the exposure station which forms a tetrahedron with the three ground points is determined by the geometrical method or the analytical method (see subsequently in

Example 1).

Refraction Correction .- Since the light ray emanating from station S2 to the camera at station S1, Fig. 6, follows the same path as the light ray traveling in the opposite direction, the observed direction of point S2 from point S1 is deviated an amount given by Eq. 43. To determine this error of refraction, the refractive constant  $\rho$  in Eq. 43 must be determined for the exposure station. As an essential requirement, at least four points, P1, P2, P3, and P4-all with known positions—must be included in one photograph. Let X, Y, and Z be the coordinates of the exposure station S calculated from the space angles  $A_{12}$ ,  $A_{13}$ , and  $A_{23}$  and the known positions of points  $P_1$ ,  $P_2$ , and  $P_3$ . From the coordinates of S, P<sub>1</sub>, P<sub>2</sub>, P<sub>3</sub>, and P<sub>4</sub> and the center of curvature C of the spheroidal surface, the direction cosines of the lines SP1, SP2, SP3, SP4, and SC may be The angles  $A_1$ ,  $A_2$ ,  $A_3$ , and  $A_4$  between the plumb line SC and the other four lines can be calculated and may be considered as their observed zenith distances, to which corrections equal to  $-\rho \tan A_1$ ,  $-\rho \tan A_2$ ,  $-\rho \tan A_3$ , and  $-\rho$  tan  $A_4$ , respectively, must be added. The correction  $a_{12}$  for the space angle  $A_{12}$  due to the corrections  $-\rho$  tan  $A_1$  and  $-\rho$  tan  $A_2$  may be found in terms of  $\rho$  by Eq. 25 on the assumption that the dihedral angle between the vertical planes SCP1 and SCP2 is not appreciably changed by the effect of refraction. Similarly, the other corrections  $a_{13}$ ,  $a_{14}$ ,  $a_{23}$ ,  $a_{24}$ , and  $a_{34}$  can all be found in terms of  $\rho$  (see subsequently in Example 6).

To account for the corrections  $a_{12}$ ,  $a_{13}$ , and  $a_{23}$ , corrections x, y, and z must be added to the calculated coordinates X, Y, and Z of the exposure station S so that Eq. 20 is satisfied. From the three equations thus obtained, the corrections x, y, and z can be found in terms of  $\rho$  (see subsequently in Example 7).

Since the corrections of the direction cosines of the lines  $SP_1$ ,  $SP_2$ , and  $SP_3$  can be found in terms of x, y, and z and therefore in terms of  $\rho$  by Eqs. 16, and  $a_{14}$ ,  $a_{24}$ , and  $a_{34}$  can all be expressed in terms of  $\rho$ , the corrections  $l_4$ ,  $m_4$ , and  $n_4$  to be applied to the direction cosines of the line  $SP_4$  may be found in terms of  $\rho$  by the three linear equations derived from Eq. 19b (see subsequently in Example 8).

The perpendicular distance  $D_4$  from point  $P_4$ , which is defined by the known coordinates  $X_4$ ,  $Y_4$ , and  $Z_4$  to line  $SP_4$  (which line, in turn, is defined by a point with coordinates (X+x), (Y+y), and (Z+z) and direction cosines  $(L_4+l_4)$ ,  $(M_4+m_4)$ , and  $(N_4+n_4)$ ) is found in terms of  $\rho$  by Eq. 18. Because of the presence of other errors of measurement, it is not likely that  $D^2_4=0$  can be satisfied by a real value of  $\rho$ ; but there is always a value of  $\rho$  for which  $D^2_4$  is minimum. Consequently,

$$\frac{d(D^2_4)}{d\rho} = 0. (44a)$$

may be used to determine  $\rho$ . Similarly if several other points  $P_5$ ,  $P_6$ , and  $P_7$  (also of known positions) appear in the same photograph, the value of  $\rho$ , determined from

$$\frac{d(D^{2}_{4} + D^{2}_{5} + D^{2}_{6} + D^{2}_{7})}{d\rho} = 0.$$
 (44b)

should be more reliable.

Equation to Determine  $\rho$ .—Let  $L_4$ ,  $M_4$ , and  $N_4$  be the direction cosines of the line which makes angles  $A_{14}$  and  $A_{24}$  with the lines  $\mathrm{SP}_1$  and  $\mathrm{SP}_2$ . When the coordinates of point S are changed to (X+x), (Y+y), and (Z+z) and the angles  $A_{14}$  and  $A_{24}$  are changed to  $(A_{14}+a_{14})$  and  $(A_{24}+a_{24})$  to account for the effect of refraction, the corrections  $x, y, z, l_4, m_4$ , and  $n_4$  may all be found in terms of  $\rho$  as illustrated subsequently by Examples 7 and 8. Since  $-D^2_4 = (X+x-X_4)^2 + (Y+y-Y_4)^2 + (Z+z-Z_4)^2 - [(L_4+l_4) \times (X+x-X_4) + (M_4+m_4) (Y+y-Y_4) + (N_4+n_4) (Z+z-Z_4)]^2$ —is an expression involving the fourth power of  $\rho$ , the expression:

$$\frac{dD^2_4}{d\rho} = 0.\dots(45)$$

gives an equation of the form:

$$a + a_1 \rho + a_2 \rho^2 + a_3 \rho^3 = 0 \dots (46)$$

the solution of which gives the value of  $\rho$ .

Orientation of Negative Plates.—As shown in Fig. 7, point S is the rear nodal point of the camera lens; X, X', Y, and Y' are the fiducial marks on the negative plate, point F is the plate intercept of a perpendicular from point S, and point V is the piercing point, on the negative plate, of the plumb line SV. Since the space angles at point S subtended by points Y,  $P_1$ , and  $P_2$  and the equations of the lines  $SP_1$  and  $SP_2$  are determinable, the equation of line SV is computed by Eqs. 26. Similarly, the equation of SF may be found also. The equation of the plumb line SV is determined from the calculated coordinates of point S and the coordinates of the center of curvature of the spheroidal surface.

The angles between the three lines SY, SF, and SV are found from their equations; and the angle YFV is found from these angles by Eq. 24. Angle FYY' can be found from the coordinates of points Y and F; and, as in the case shown, the angle between the lines FV and YY', is equal to

$$\angle \overline{FIY'} = \angle \overline{FYY'} + \angle \overline{YFV} \dots (47)$$

Since the equation of the plane of SV and SF can be determined from the equations of the two lines, and the equation of the plane of the meridian through point S is found from Eq. 5, the azimuth of line SF is found from the dihedral angle between these two planes.

The coordinates of point F, the azimuth of line SF, and angles VSF and FIY' completely define the orientation of the negative plate at the instant

of exposure.

As shown in Fig. 8, points  $S_1$  and  $S_2$  are two exposure stations and  $V'_1$  and  $V'_2$  represent the feet of the plumb lines on the spheroidal surface. The elevations  $V'_1S_1$  and  $V'_2S_2$  can be found from the calculated coordinates of points  $S_1$  and  $S_2$  by Eq. 1. The azimuth of the line  $S_1S_2$  is found from the dihedral angle between the meridian plane of point  $S_1$  and the vertical plane containing

points S1 and S2.

Since the equations of lines  $S_1S_2$  and  $S_1V'_1$  and the plate perpendicular  $S_1F_1$  can be found, it is possible to compute the angle  $\nu_1$ , Fig.  $\dot{S}$ , which may be considered as the vertical angle of the line of sight of the camera, and the angle  $V'_1S_1S_2$ . The horizontal angle  $\beta_1$  is determined from the three space angles at point  $S_1$  by Eq. 24. Similarly, the vertical and horizontal angles of the plate perpendicular  $S_2F_2$  at point  $S_2$  are found. These angles, together with the angles of orientation of the two negative plates, completely define their relative position in the air.

Position of Photographed Point.—The space angles at the exposure station  $S_1$  made by a point  $P_n$  of unknown position with known points  $P_1$ ,  $P_2$ , and  $P_3$  can be determined from their images on the negative plate by Eqs. 7 and they can be corrected for atmospheric refraction by Eq. 25, when the position of station  $S_1$  and the refractive constant  $\rho$  are known. From these space angles the direction cosines of the line  $S_1P_n$  may be determined from Eqs. 26 or 32 according to the number of known points available.

Similarly, if point  $P_n$  is also photographed at another exposure station  $S_2$  and the equation of the line  $S_2P_n$  can be determined, the accuracy of all the data may be tested by computing the perpendicular distance between the two lines  $S_1P_n$  and  $S_2P_n$  with Eq. 21a. If this distance is reasonably small, its midpoint may be considered as the probable position of point  $P_n$  and its coordinates may be determined by Eqs. 23.

Four or More Points of Known Positions in One Photograph.—In this case more data are available than necessary and the method of least squares is used to determine the coordinates of the exposure station S.

The coordinates of point S are first calculated from the space angles of points  $P_1$ ,  $P_2$ , and  $P_3$ . Then the lengths and direction cosines of lines  $SP_1$ ,  $SP_2$ ,  $SP_3$ , and  $SP_4$  are calculated; and the small corrections  $a_{12}$ ,  $a_{13}$ , and  $a_{23}$  of the space angles  $A_{12}$ ,  $A_{13}$ , and  $A_{23}$  due to corrections x, y, and z which must be added to the calculated coordinates of point S are found from Eq. 20.

Let the values of angles  $A_{14}$ ,  $A_{24}$ , and  $A_{34}$ , calculated from the direction cosines of the four lines, be denoted by  $I_{14}$ ,  $I_{24}$ , and  $I_{34}$  and their increases in terms of x, y, and z, found by Eq. 20, be denoted by  $i_{14}$ ,  $i_{24}$ , and  $i_{34}$ , respectively. The corrections  $a_{14}$ ,  $a_{24}$ , and  $a_{34}$  when added to the observed values  $A_{14}$ ,  $A_{24}$ ,

and A34, respectively, must yield

$$I_{14}+i_{14}=A_{14}+a_{14} \qquad \qquad (48a)$$
 
$$I_{24}+i_{24}=A_{24}+a_{24} \qquad \qquad (48b)$$
 and 
$$I_{34}+i_{34}=A_{34}+a_{34} \qquad \qquad (48c)$$
 Therefore, 
$$a_{14}=I_{14}-A_{14}+i_{14} \qquad \qquad (49a)$$
 
$$a_{24}=I_{24}-A_{24}+i_{24} \qquad \qquad (49b)$$
 and 
$$a_{34}=I_{34}-A_{34}+i_{34} \qquad \qquad (49c)$$

which give the angular corrections  $a_{14}$ ,  $a_{24}$ , and  $a_{34}$ , also, as linear functions of x, y, and z.

As it is possible to restrain the angular corrections if the sum Q, defined by the condition that—

$$Q = W_{12} (a_{12})^2 + W_{13} (a_{13})^2 + W_{23} (a_{23})^2 + W_{14} (a_{14})^2 + W_{24} (a_{24})^2 + W_{34} (a_{34})^2..(50)$$

—shall be a minimum, the values of x, y, and z can be determined from the three normal equations derived from  $\frac{dQ}{dx} = 0$ ,  $\frac{dQ}{dy} = 0$ , and  $\frac{dQ}{dz} = 0$ , respectively (see subsequently in Example 5). In Eq. 50, W is the weight of a corresponding observed value. The foregoing method can be extended to other cases involving five or six ground points of known positions.

Four or More Points of Known Positions and One or Several Points of Unknown Positions Common to Two Aerial Photographs.—Let the positions of four ground points  $P_1$ ,  $P_2$ ,  $P_3$ , and  $P_4$  be given, with that of  $P_5$  undetermined, and let the available data be composed of the space angles  $A_{12}$ ,  $A_{13}$ ,  $A_{14}$ ,  $A_{23}$ ,  $A_{24}$ ,  $A_{34}$ ,  $A_{15}$ ,  $A_{25}$ , and  $A_{35}$  and  $A'_{12}$ ,  $A'_{13}$ ,  $A'_{14}$ ,  $A'_{23}$ ,  $A'_{24}$ ,  $A'_{34}$ ,  $A'_{15}$ ,  $A'_{25}$ , and  $A'_{35}$  at the exposure stations  $P_5$  and  $P_5$  and  $P_5$  are first computed from the given coordinates of points  $P_5$ ,  $P_5$ , and  $P_5$  by the use of the space angles at  $P_5$  and  $P_5$  are first computed from the given coordinates of points  $P_5$ ,  $P_5$ , and  $P_5$  by the use of the space angles at  $P_5$  and  $P_5$  are first computed from the given coordinates of points. Then, corrections to these coordinates  $P_5$ ,  $P_5$ , and  $P_5$ , respectively, and other corrections are computed as explained in the following text.

For the points of known positions, corrections  $a_{12}$ ,  $\dot{a}_{13}$ ,  $a_{23}$ ,  $a_{14}$ ,  $a_{24}$ ,  $a_{34}$ ,  $a'_{12}$ ,  $a'_{13}$ ,  $a'_{23}$ ,  $a'_{14}$ ,  $a'_{24}$ , and  $a'_{34}$  of their space angles may be expressed in terms of x, y, and z and x', y', and z', respectively, by Eq. 20. For point  $P_5$  (position unknown), the direction cosines of lines  $SP_5$  and  $S'P_5$  are found from their space angles fixed by three points of known positions such as  $P_1$ ,  $P_2$ , and  $P_3$  by Eqs. 32; and their corrections  $l_5$ ,  $m_5$ ,  $n_5$ ,  $l'_5$ ,  $m'_5$ , and  $n'_5$  are also to be determined. The correction  $a_{15}$  may be expressed in terms of  $l_1$ ,  $m_1$ ,  $n_1$ ,  $l_5$ ,  $m_5$ , and  $n_5$  by Eq. 19b;  $l_1$ ,  $m_1$ , and  $n_1$  can be obtained in terms of x, y, and z; and  $n_5$  can be found in terms of  $l_5$  and  $m_5$  by Eq. 17. Therefore,  $a_{15}$  can be expressed in terms of x, y, z,  $l_5$ , and  $m_5$ . Similarly,  $a_{25}$  may be expressed in terms of the same five corrections. The correction  $a_{35}$  should be determined from  $a_{12}$ ,  $a_{13}$ ,  $a_{23}$ ,  $a_{15}$ , and  $a_{25}$  by Eq. 30 and therefore may also be expressed in terms of the

foregoing five corrections. Likewise  $a'_{15}$ ,  $a'_{25}$ , and  $a'_{35}$  may be expressed in terms of x', y', z',  $l'_{5}$ , and  $m'_{5}$ .

To make the two lines SP<sub>5</sub> and S'P<sub>5</sub> intersect, however, corrections  $x, y, z, x', y', z', l_5, m_5, l'_5$ , and  $m'_5$  must satisfy a conditional equation based on Eq. 22 in which  $n_5$  and  $n'_5$  are replaced by their equivalents derived from Eqs. 16. By this condition of intersection, one of the corrections such as  $m'_5$  can be found in terms of the other nine.

The foregoing analysis shows that the basic corrections in this case are x, y, z, x', y', z',  $l_5$ ,  $m_5$ , and  $l'_5$  which may be determined from a set of normal equations if the sum of the weighted squares of the corrections of the space angles is a minimum. The method is general and applicable to the case with (3+q)-points of known positions and u points of unknown positions; but the number of basic corrections, being equal to 3(2+u) is dependent on u only. The method can also be modified to include such data as  $A_{45}$  and  $A'_{45}$ . If the refractive constant  $\rho$  is known, the space angles should first be corrected for refraction by Eq. 25, and then they should be used as the data to start the aforementioned method of analysis.

If (because of some special arrangement to be explained subsequently) the position of the exposure station S has been determined previously, the corrections x, y, and z may be considered equal to zero. The number of the basic corrections is then decreased by 3, thus decreasing

the work of numerical calculation greatly.

Analysis of Partly Overlapping Aerial Survey.— Fig. 9 represents an aerial survey with a 60% overlap between photographs G<sub>1</sub>, G<sub>2</sub>, and G<sub>3</sub>. The method of successive approximation can be applied to cases of this kind. It is assumed at the beginning that: (1) At least three points of known positions, such as

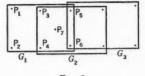


Fig. 9

 $P_1$ ,  $P_3$ , and  $P_4$ , are available for the determination of the exposure station  $S_1$  at which photograph  $G_1$  is taken; (2) in photograph  $G_2$  only two points of known positions such as  $P_3$  and  $P_4$  can be identified; and (3) each of the succeeding photographs contains at least two well-defined points in common with two previous photographs. It is also assumed that: (4) The angles of tilt are small and (5) the altimeter records are reasonably reliable. Under these assumed conditions, the exposure station may be represented approximately, in plan, by the principal point, and in elevation by the altimeter reading. If the principal point of a vertical photograph can be identified in a previous photograph by its surrounding topographic details, its position in plan is estimated or determined by the radial-line method of plotting.

From the known coordinates of points  $P_3$  and  $P_4$ , and from the estimated coordinates of the exposure station  $S_2$  at which photograph  $G_2$  is taken, the direction cosines of the lines  $S_2P_3$  and  $S_2P_4$  and their including angle  $I''_{34}$  may be calculated. The change  $i''_{34}$  of  $I''_{34}$  due to corrections  $x_2$ ,  $y_2$ , and  $z_2$  to be added to the estimated coordinates  $X_2$ ,  $Y_2$ , and  $Z_2$  of point  $S_2$  may be found by Eq. 20. Since the sum  $(I''_{34} + i''_{34})$  must be equal to the observed value  $A''_{34}$  of the space angle at point  $S_2$ ,

$$i''_{34} = A''_{34} - I''_{34} \dots (51)$$

which is a linear equation in  $x_2$ ,  $y_2$ , and  $z_2$ . If it is believed that  $x_2$  and  $y_2$  are relatively small because  $X_2$  and  $Y_2$  are more reliably determined than  $Z_2$ , Eq. 51 can be used to find  $z_2$  by omitting the terms involving  $x_2$  and  $y_2$ . In that case, the calculation should be begun again with  $X_2$ ,  $Y_2$ , and  $(Z_2 + z_2)$  as the initial values. It is essential that good initial values be obtained by a simple method so that the entire process of calculation can be shortened.

The direction cosines of line  $S_2P_5$  may be calculated from the computed values of the direction cosines of lines  $S_2P_3$  and  $S_2P_4$  and the space angles  $A^{\prime\prime}_{25}$  and  $A^{\prime\prime}_{45}$  by Eqs. 26. The changes in the direction cosines of lines  $S_2P_3$  and  $S_2P_4$  due to the corrections  $x_2$ ,  $y_2$ , and  $z_2$ , and the corrections  $l^{\prime\prime}_5$ ,  $m^{\prime\prime}_5$ , and  $n^{\prime\prime}_5$  to be applied to the calculated values of the direction cosines of line  $S_2P_5$  must not alter the values of the space angles  $A^{\prime\prime}_{35}$  and  $A^{\prime\prime}_{45}$ . Therefore,

$$L''_{5}l''_{3} + L''_{3}l''_{5} + M''_{5}m''_{3} + M''_{3}m''_{5} + N''_{5}n''_{3} + N''_{3}n''_{5} = 0..(52a)$$

$$L''_{5}l''_{4} + L''_{4}l''_{5} + M''_{5}m''_{4} + M''_{4}m''_{5} + N''_{5}n''_{4} + N''_{4}n''_{5} = 0..(52b)$$

and

in which all the capital letters denote numerical constants determined from known or assumed values. Corrections  $l''_3$ ,  $m''_3$ ,  $n''_3$ ,  $l''_4$ ,  $m''_4$ , and  $n''_4$  can be found in terms of  $x_2$ ,  $y_2$ , and  $z_2$  by Eqs. 16; therefore corrections  $l''_5$ ,  $m''_5$ , and  $n''_5$  can also be obtained in terms of  $x_2$ ,  $y_2$ , and  $z_2$  by Eqs. 16. The line  $S_2P_5$  must intersect line  $S_1P_5$ , however; and the equation of the latter has been determined completely in the analysis of the previous survey. Hence, the equation of intersection based on Eq. 22 involves only  $x_2$ ,  $y_2$ ,  $z_2$ ,  $l''_5$ ,  $m''_5$ , and  $n''_5$  as the unknowns; and, as the latter three corrections may be found in terms of the corrections of coordinates, the final result is again a linear equation in  $x_2$ ,  $y_2$ , and  $z_2$ . Similarly, the intersection of the lines  $S_1P_6$  and  $S_2P_6$  gives another equation in  $x_2$ ,  $y_2$ , and  $z_2$ . These three equations are used to determine the values of the three corrections.

As the errors of the initial values  $X_2$ ,  $Y_2$ , and  $Z_2$  may be very large, the corrected values  $(X_2 + x_2)$ ,  $(Y_2 + y_2)$ , and  $(Z_2 + z_2)$  are usually only better approximations. With these revised values of the coordinates of point  $S_2$  as the new base for starting, the foregoing procedure is repeated (see subsequently in Example 9).

Since there may be other well-defined points such as  $P_7$  common to photographs  $G_1$  and  $G_2$ , and since from each common point a linear equation in  $x_2$ ,  $y_2$ , and  $z_2$  can be derived, more equations may be obtained than are required for the determination of the three unknowns. In such a case, in the second course of approximation, the method of least squares is used.

After the coordinates of point  $S_2$  are thus calculated, the equations of the lines from point  $S_2$  to all points in photograph  $G_2$  may be computed, and the positions of the points common to photographs  $G_1$  and  $G_2$  may be determined by solving Eqs. 23.

The same method of calculation may be used for obtaining the positions of the succeeding exposure stations and points, until the survey ends or closes on another point of known position. Aerial Triangulation.—Present practice in aerial surveying is mainly based on the use of partly overlapping (generally 60%) exposures as shown in Fig. 9. The method of computing such a survey is not straightforward although results may be obtained which are as reliable as the angular data.

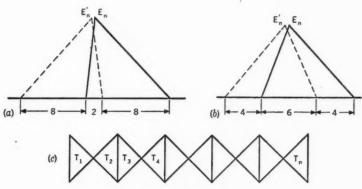


Fig. 10.—Aerial Triangulation

A fully duplicated, and also partly overlapping, aerial survey may be made with two cameras so arranged that their fields of view have a longitudinal overlap of 20% as shown in Fig. 10(a) or of 60% as shown in Fig. 10(b). When cameras arranged in such a combination are operated simultaneously by electric control so that the exposures of each camera have a 60% overlap of their own, each photograph of one camera will be nearly duplicated by some photograph of another.

Let a series of triangles  $T_1, T_2, \dots, T_n$  on a strip of ground surface (see Fig. 10(c)) be so spaced that adjacent pairs will appear together in the same

TABLE 1A.—Schedule of Sequence in the Use of Triangles in Aerial Triangulation Computations;

25% Overlap

Exposures	Backsights	Foresights
E <sub>1</sub> E'3	T <sub>1</sub> T <sub>1</sub>	T <sub>2</sub> T <sub>2</sub>
E <sub>2</sub> E' <sub>4</sub>	$\frac{\mathrm{T}_2}{\mathrm{T}_2}$	T <sub>2</sub> T <sub>3</sub>
E <sub>n</sub> E' <sub>n+2</sub>	T <sub>n</sub> T <sub>n</sub>	$T_{n+1}$ $T_{n+1}$

TABLE 1B.—Schedule of Sequence in the Use of Triangles in Aerial Triangulation Computations;
60% Overlap

Exposures	Backsights	Foresights					
E <sub>1</sub> E' <sub>2</sub>	T <sub>1</sub> T <sub>1</sub>	T <sub>2</sub> T <sub>2</sub>					
E <sub>2</sub> E' <sub>3</sub>	$T_2$ $T_2$	T <sub>2</sub> T <sub>3</sub>					
E <sub>n</sub> E' <sub>n+1</sub>	T <sub>n</sub> T <sub>n</sub>	$T_{n+1}$ $T_{n+1}$					

photograph; and let the simultaneous exposures of the two cameras at the jth exposure station be denoted by  $E_i$  and  $E'_i$ . If triangle  $T_1$  is composed of points of known positions and may be used to determine the position of the exposure

station, the two sets of exposures can be compiled as in Tables 1A and 1B, showing the sequence in which calculations are made. As in the case of a ground survey, the sights used to determine the positions of the instrument are called the "backsights," and those used to determine the points sighted are called the "foresights." In either case the backsights of triangle  $T_1$  are used to find the positions of the exposure stations  $S_1$  and  $S'_2$  or  $S_1$  and  $S'_2$  and the foresights from two stations of known positions are used to find the vertexes of triangle  $T_2$ . This process of calculation is continued until the series of triangles ends on another point of known position.

As the relative position of the exposure stations  $S_n$  and  $S'_n$  is identical to that of the two cameras in the airplane, when their positions are determined independently, additional checks on the accuracy of the data and the numerical

calculations are provided by the spacing of the cameras. Error of Closure of Aerial Triangulation.—When an aerial survey begins from a ground triangle composed of points of known positions and ends on another known ground point, the coordinates  $X_a$ ,  $Y_a$ , and  $Z_a$  of the terminal point determined by the aerial survey should agree with the more reliable values  $X_g$ ,  $Y_g$ , and  $Z_g$  determined by the ground survey. The errors in three directions are:  $e_x = X_a - X_g$ ,  $e_y = Y_a - Y_g$ , and  $e_z = Z_a - Z_g$ , respectively, and the error of closure is equal to  $e_c = \sqrt{e^2x + e^2y + e^2z}$ . Let the distance between one of the starting points and the terminal point be D. The ratio  $\frac{e}{D}$ , reduced to the form  $\frac{1}{N}$ , will serve as a measure of the accuracy of the survey.

If it is assumed that the errors  $e_x$ ,  $e_y$ , and  $e_z$  are accumulated equally from all parts of the survey, the corrections to be added to the calculated coordinates of an intermediate point should be:  $\frac{-e_z}{D}D_i$ ,  $\frac{-e_y}{D}D_i$ , and  $\frac{-e_z}{D}D_i$ , respectively.

TABLE 2.—COORDINATES OF GROUND POINTS, IN METERS (1 Meter = 3.28083 Ft)

Point	X	Y	Z
P <sub>1</sub>	2,578.45	3,876.38	68.49
	5,769.38	1,643.21	135.27
P4	6,043.75	5,872.15	146.73
	2,385.00	5,934.00	76.30

## ILLUSTRATIVE EXAMPLES

starting point.

tively, in which  $D_i$  is the dis-

tance of the point from the

Data.—The data in Table 2 are assumed for the purpose of making sample calculations that will illustrate the general features of the methods and offer

some indication as to the accuracy of the results to be expected. The positions of two exposure stations are also assumed so that the field angles to the various ground points are, for station  $S_1$ ,

Angle																		Value
$P_1S_1P_2$													*				67°	· car cro
$P_1S_1P_3$																	$69^{\circ}$	47'
$P_2S_1P_3$																	75°	38'
$P_1S_1P_4$														*			$29^{\circ}$	08.011'
$P_2S_1P_4$																	89°	53.239'
P.S.P.																	55°	47 648'

and, for station S2,

Angle	Value
$P_1S_2P_2$	44° 04.508′
$P_1S_2P_3$	
$P_1S_2P_4$	29° 24.033′
$P_2S_2P_3$	49° 10.383′
$P_2S_2P_4$	68° 52.333′

Example 1. To Find the Coordinates  $X_1$ ,  $Y_1$ , and  $Z_1$  of Exposure Station  $S_1$ .— The lengths  $D_1$ ,  $D_2$ , and  $D_3$  of lines  $S_1P_1$ ,  $S_1P_2$ , and  $S_1P_3$  must first be determined from the equations:

$$D^{2}_{1} - 2(0.3824147) D_{1} D_{2} + D^{2}_{2} - 15,173,542 = 0......(53a)$$

and

$$D_{2}^{2} - 2(0.2481263) D_{2} D_{3} + D_{3}^{2} - 17,959,339 = 0......(53c)$$

The results are summarized in Table 3. The final values are accurate to the

TABLE 3.—Computation of Lengths D by Eqs. 53 (In Meters)

	D	1	D	<b>'</b> 2	D	's	D"2	$d_1$
Trial No.	Assumed (1)	Corrected (2)	Computed (3)	Corrected (4)	Computed (5)	Corrected (6)	Computed (7)	Computed (8)
1 2 3			+3,504.87 +3,463.818 +3,464.916	+3,465.27 +3,464.918 +3,464.910	+3,487.09 +3,445.691 +3,446.796	+3,447.09 +3,446.791 +3,446.791	+3,424.18 +3,466.005 +3,464.904	+39.6 - 1.043 + 0.0057

third decimal place. The coordinates of point  $S_1$  (in meters) are then found to be:  $X_1 = 4,953.549$ ,  $Y_1 = 3,827.388$ , and  $Z_1 = 2,698.353$ .

Example 2. To Find the Elevation of Station  $S_1$ .—Let the radius of curvature of the spheroidal surface be R=6,363,200 meters (1 meter = 3.28083 ft).

It is obvious that the third term in Eq. 1 can be neglected in this case and that the elevation  $P_fS_1$ , therefore, is  $2,698.353 + \frac{19,593,000}{6,365,900} = 2,701.431 \text{ m}.$ 

Example 3. To Find the Direction Cosines of Lines  $S_1P_f$ ,  $S_1P_1$ ,  $S_1P_2$ ,  $S_1P_3$ ,  $S_1P_4$ , and the Included Angles.— The direction cosines of the plumb TABLE 4.—Computation of Direction Cosines

Line	L	M	N
$S_1P_1$	-0.0007781	-0.0006012	-0.9999995
$S_1P_1$	-0.6701815	+0.0138241	-0.7420682
S1P2	+0.2354551	-0.6303707	-0.7397255
$S_1P_3$	+0.3162945	+0.5932365	-0.7402895
S1P4	-0.6069250	+0.4977735	-0.6195673

line  $S_1P_f$  are determined from the coordinates of station  $S_1$ , and the center of curvature (X=0, Y=0, and Z=-6,363,200). Similarly, the direction cosines of the other four lines may be found from the known coordinates (see Table 4). From the direction cosines the following space angles may be com-

puted by Eq. 19a:

Angle																	V	alue
$P_fS_1P_1$ .																	42° (	2.893
$P_fS_1P_2$ .																		
PfS1P3.																	42° 1	7.720
PfS1P4.																		

and, as measured to P4,

Angle	Value
$P_1S_1P_4$	 29° 08.681′
$P_2S_1P_4$	 89° 54.417′
$P_3S_1P_4$	 55° 48.389′

Example 4. To Find the Direction Cosines of Line S<sub>1</sub>P<sub>4</sub> from the Direction Cosines of Lines S<sub>1</sub>P<sub>1</sub>, S<sub>1</sub>P<sub>2</sub>, and S<sub>1</sub>P<sub>3</sub>.—The direction cosines of line S<sub>1</sub>P<sub>4</sub> found

TABLE 5.—DIFFERENCES BETWEEN
THE ASSUMED AND COMPUTED
VALUES OF ANGLES AT
STATION S<sub>1</sub>

Angle	Assumed	Computed	Error
P <sub>1</sub> S <sub>1</sub> P <sub>4</sub>	29° 08.011′	29° 08.681′	0° 00.670′
	89° 53.239′	89° 54.417′	0° 01.178′
	55° 47.648′	55° 48.389′	0° 00.741′

from the calculated values of angles  $P_1S_1P_4$ ,  $P_2S_1P_4$ , and  $P_3S_1P_4$  by Eqs. 32 are:  $L_4=-0.6069253$ ,  $M_4=+0.4977740$ , and  $N_4=-0.6195673$ .

Example 5. To Find the Most Probable Position of S<sub>1</sub>.—Table 5 demonstrates the differences between the assumed and the calculated values of the angles P<sub>1</sub>S<sub>1</sub>P<sub>4</sub>, P<sub>2</sub>S<sub>1</sub>P<sub>4</sub>, and P<sub>3</sub>S<sub>1</sub>P<sub>4</sub>. If these errors are to be distributed to the six space angles at

station  $S_1$  by introducing corrections  $x_1$ ,  $y_1$ , and  $z_1$  to the calculated coordinates of station  $S_1$ , it may be found from Eq. 20 that

and Eqs. 20 and 49 will yield

and

$$a_{14} = 0.670 - 0.27669 x_1 + 0.26535 y_1 - 0.27829 z_1 \dots (55a)$$

$$a_{24} = 1.178 - 0.41046 x_1 - 0.01784 y_1 - 1.2136 z_1 \dots (55b)$$

$$a_{34} = 0.741 - 0.30059 x_1 + 0.50608 y_1 - 0.63048 z_1 \dots (55c)$$

The angular corrections are all reduced to minutes of arc by multiplying the results by 3,437.7, which is the number of minutes in one radian. The normal equations derived from these corrections, with equal weights, are

from which  $x_1 = 0.9105$ ,  $y_1 = -0.3851$ , and  $z_1 = 0.2258$ . These are the corrections to be added algebraically to the coordinates of  $S_1$ , respectively.

Example 6. To Find the Corrections of the Space Angles for Atmospheric Refraction.—By using the calculated values of the space angles between the lines  $S_1P_f$ ,  $S_1P_3$ ,  $S_1P_3$ ,  $S_1P_3$ , and  $S_1P_4$ , Eq. 25 yields:  $a_{12}=-1.3368~\rho$ ,  $a_{13}=-1.3948~\rho$ ,  $a_{23}=-1.5523~\rho$ ,  $a_{14}=-0.5873~\rho$ ,  $a_{24}=-2.0283~\rho$ , and  $a_{34}=-1.0970~\rho$ —in which  $\rho$  is in minutes.

Example 7. To Find the Corrections of the Coordinates of  $S_1$  in Terms of  $\rho$ .—By equating the errors of refraction in angles  $P_1SP_2$ ,  $P_1S_1P_3$ , and  $P_2S_1P_3$  to those due to the corrections  $x_1$ ,  $y_1$ , and  $z_1$  to be added to the calculated coordinates of station  $S_1$ , three equations (in meters)—

0.30007 
$$x_1 + 0.39364 y_1 + 0.97174 z_1 = 1.3368 \rho \dots (57a)$$
  
0.26216  $x_1 - 0.40511 y_1 + 1.0170 z_1 = 1.3945 \rho \dots (57b)$   
 $-0.42573 x_1 + 0.03279 y_1 + 1.1427 z_1 = 1.5523 \rho \dots (57c)$ 

—may be obtained which give:  $x_1 = 0.0231 \ \rho$ ,  $y_1 = 0.0042 \ \rho$ , and  $z_1 = 1.3669 \ \rho$ .

Example 8. To Find the Corrections of the Direction Cosines in Terms of  $\rho$ .—

From the values of  $x_1$ ,  $y_1$ , and  $z_1$  found in Example 7 and from other known values, by Eqs. 16, the corrections of the direction cosines of  $S_1P_1$ ,  $S_1P_2$ , and  $S_1P_3$  may be determined as shown in Table 6 in which  $l_4$ .  $m_4$ , and  $n_4$  are found from the three linear equations derived from Eq. 19b.

TABLE 6.—Corrections for Direction Cosines, Example 8

Line	ı	m	n
S <sub>1</sub> P <sub>1</sub> S <sub>1</sub> P <sub>2</sub> S <sub>1</sub> P <sub>3</sub> S <sub>1</sub> P <sub>4</sub>	$^{+0.0001882}_{-0.0000752}$ $^{\rho}_{-0.0000985}$ $^{\rho}_{-0.0001706}$	$\begin{array}{c} -0.0000057 \ \rho \\ +0.0001822 \ \rho \\ -0.0001737 \ \rho \\ -0.0001457 \ \rho \end{array}$	$ \begin{array}{c} -0.0001701 \ \rho \\ -0.0001792 \ \rho \\ +0.0001813 \ \rho \\ -0.0002847 \ \rho \end{array} $

Example 9. To Find the Coordinates of Station  $S_2$  from Space Angles, and the Known Positions of Points  $P_1$  and  $P_2$ .—The initial values assumed for the coordinates of station  $S_2$  are:  $X_2 = 4,400$ ,  $Y_2 = 6,210$ , and  $Z_2 = 3,160$ . With

TABLE 7.—COORDINATES OF STATION S2 COMPUTED FROM SPACE ANGLES

	$X_2$			$Y_2$		Z <sub>2</sub>						
As- sumed	Corrections	Corrected (3)	As- sumed	Corrections	Corrected (6)	As- sumed	Corrections	Corrected (9)				
+4.400	-58.5	+4,341.5	+6,210	-69.7	+6,140.3	+3,160	-19.8	+3,140.2 +3,131.8				
	(1) -4.400	As- sumed Correc- tions (1) (2)	As- sumed corrected  (1) (2) (3)  +4,400 -58.5 +4,341.5 +4,341 +17.6 +4,358.6	As- sumed ctions Corrected Sumed  (1) (2) (3) (4)  +4,400 -58.5 +4,341.5 +6,210 +4,341 +17.6 +4,358.6 +6,140	As- sumed tions	As- sumed corrected sumed corrected from tions corrected  (1) (2) (3) (4) (5) (6)  +4,400 -58.5 +4,341.5 +6,210 -69.7 +6,140.3 +6.130 +10.4 +6.150.4	As- sumed tions  (1) (2) (3) (4) (5) (6) (7)  +4,400 -58.5 +4,341.5 +6,210 -69.7 +6,140.3 +3,160 +4,341 +17.6 +4,358.6 +6,140 +10.4 +6,150.4 +3.140	As- sumed Corrected Sumed Su				

these and other known values the solution of Eq. 51 yields:  $536.9 x_2 - 2,160.7 y_2 - 1,499.2 z_2 = 150,214$ , as a correction for angle  $P_1S_2P_2$ . Similarly, Eq. 22 yields

$$0.1691 x_2 + 0.1015 y_2 + 0.5552 z_2 = -28.02$$
$$0.2796 x_2 - 0.5559 y_2 - 0.4415 z_2 = 31.12.$$

and

and

for making lines  $S_2P_3$  and  $S_2P_4$  intersect the lines  $S_1P_3$  and  $S_1P_4$ , respectively. The solution of these three equations gives:  $x_2 = -58.5$ ,  $y_2 = -69.7$ , and  $z_2 = -19.8$ . The foregoing results and those of a further process of approximation are shown in Table 7 which indicates a steady convergence to the true values.

### ACKNOWLEDGMENT

In the final editing stage, when publication promised to be greatly delayed by difficulties of long-distance communication, Mr. Underwood very kindly served as the writer's "proxy," thus advancing the release of this paper.



## AMERICAN SOCIETY OF CIVIL ENGINEERS

OFFICERS FOR 1950

PRESIDENT ERNEST E. HOWARD

### VICE-PRESIDENTS

Term expires January, 1951: HENRY J. SHERMAN ROBERT B. BROOKS

Term expires January, 1952: FRED C. SCOBEY ALBERT HAERTLEIN

#### DIRECTORS

Term expires January, 1951: Term expires January, 1952: Term expires January, 1953:

KIRBY SMITH JOEL D. JUSTIN FRANCIS S. FRIEL WEBSTER L. BENHAM EDMUND FRIEDMAN C. GLENN CAPPEL

WILLIAM M. GRIFFIN WALDO G. BOWMAN KIRBY SMITH WORRIS GOODKIND HAROLD L. BLAKESLEE PAUL L. HOLLAND S. T. HARDING

OTTO HOLDEN GROOM H. BUTLER
GORDON H. BUTLER
LOUIS R. HOWSON
G. BROOKS EARNEST
WALTER J. RYAN
GEORGE W. LAMB

## PAST-PRESIDENTS

Members of the Board

R. E. DOUGHERTY

TREASURER CHARLES E. TROUT

ASSISTANT TREASURER GEORGE W. BURPEE

FRANKLIN THOMAS

EXECUTIVE SECRETARY WILLIAM N. CAREY

ASSISTANT SECRETARY E. L. CHANDLER

## PROCEEDINGS OF THE SOCIETY

SYDNEY WILMOT Manager of Technical Publications

HAROLD T. LARSEN Editor of Technical Publications

COMMITTEE ON PUBLICATIONS WALDO G. BOWMAN

FRANCIS S. FRIEL S. T. HARDING

OTTO HOLDEN LOUIS R. HOWSON

KIRBY SMITH